WETLAND PROTECTION ACT

MASSACHUSETTS G.L. C. 131 s. 40

NOTICE OF INTENT

Rec'd 2-3-78

ALL PARTS OF THIS FORM SHALL BE COMPLETED UNDER THE PAINS AND PENALTY OF PERJURY INCLUDING THE ENVIRONMENTAL DATA FORM ATTACHED HERETO AND MADE A PART HEREOF OR THE FILING MAY BE CONSIDERED INCOMPLETE

DATE February 1, 1978

1. Notice is hereby given in accordance with the provisions of G.L. C. 131 s. 40, that the proposed activity as submitted herein falls within the jurisdiction of said statute, and is located in the

City/Town Acton Arborwood Street.

Recorded at the Registry of Middlesex South, Book 9900, Page 173 174.

Certificate (if registered) n.a.

2. The name(s), address and telephone number of owner(s) of the subject land are:

NAME	ADDRESS	TELEPHONE NO.
Boudreau Family Trust		
Boudreau Family Trust	41 Baskin Road, Lexington, Mass	862-0858

3. I have filed identical Notices and plans by Certified Mail with the following:

Conservation Commission (original) Date February 1, 1978

Mass. Department of Environmental Quality
75B Grove Street
Worchester, Mass. 01605 Date February 1, 1978

4. Have all permits, variances and approvals as required by C. 131 s. 40 been obtained? yes* (yes or no). Attach copies to original Notice of Intent.
*Subdivision approval appliedfor.

5. Enclosed is Statutory Filing Fee of $25.00, payable to City or Town.

6. The name, address and telephone number of the owner's representatives (if any) are as follows:

	NAME	ADDRESS	TELEPHONE NO.
Attorney			
Engineer	Joseph W. Moore Co.	16 Railroad Ave., Bedford, Mass.	275-7979
Agent	Northeast structures and Development Inc.	P.O. Box 215, Bedford, Mass.	263-7305

7. The purpose of the project is: (Use additional sheets if necessary)
Construct 5 lot subdivision.

8. Owner's (or agent's signature) Ernest J. DeVincent Pres.

"B"

ARBORWOOD HIGHLANDS
USGS LOCUS MAP
1"=2000'

WETLAND PROTECTION ACT

ENVIRONMENTAL DATA FORM

1. All parts of this form are to be filled out by the applicant or his agent under the provisions of G.L. C. 131 s. 40.
2. Where a section is not relevant to the application in question, the words "Not Applicable" should be entered on the appropriate line.

NAME OF APPLICANT: Northeast Structures and Development Inc.

ADDRESS OF APPLICANT: P.O. Box 215, Bedford Mass. 01730

MUNICIPALITIES WHERE ACTIVITY IS PROPOSED AND NOTICE IS FILED: Acton

DESCRIPTION OF PROPERTY INVOLVED IN APPLICATION (including the dimensions of any existing buildings, decks, marinas, existing cesspools): Knoll surrounded by unnamed brooks.

DESCRIPTION OF MODIFICATIONS PROPOSED ON THE SITE, including grading, dredging, removal of vegetation, etc.: Grading for roadway, house, and drive construction.

A. SOILS

1. United States Department of Agriculture Soil Types (show on map): Hinckley

2. Permeability of soil on the site. (Dates of testing)

   No tests.

3. Rate of percolation of water through the soil. (Dates of testing) 12/28/77

   Rates from 0.7 to 4.0 min./inch

B. SURFACE WATERS

1. Distance of site from nearest surface water (Date of measurement): at boundary

2. Sources of runoff water	Lightly developed residential area.
3. Rate of runoff from the site	100 year storm at time of peak receiving brook. Existing= 2.14cfs Proposed= 2.65cfs
4. Destination of runoff water	Coles Brook
5. Chemical additives to runoff water on the site	none

C. GROUND COVER

1. Extent of existing impervious ground cover on the site	0%
2. Extent of proposed impervious ground cover on the site	11.5%
3. Extent of existing vegetative cover on the site	100%
4. Extent of proposed vegetative cover on the site	88.5%

D. TOPOGRAPHY

1. Maximum existing elevation on site	186
2. Minimum existing elevation on site	150
3. Maximum proposed elevation of site	186
4. Minimum proposed elevation of site	150
5. Description of proposed change in topography	Grading for roads, houses and drives.

E. GROUND WATER

1. Minimum depth to water table on site (at time of filing)	6.5 feet
2. Maximum depth to water table on site (at time of filing)	over 10feet
3. Seasonal maximum ground water elevation	6.5 feet

F. WATER SUPPLY	
1. The source of the water to be provided to the site	Town water
2. The expected water requirements (g.p.d.) for the site	2200gpd
3. The uses to which water will be put	Domestic
G. SEWAGE DISPOSAL	
1. Sewage disposal system (description and location on the site, of system)	On site disposal systems located at test pits shown on the plans.
2. Expected content of the sewage effluents (human waste, pesticides, detergents, oils, heavy metals, other chemicals)	Human waste Human Waste
3. Expected daily volume of sewage	2000 gpd
H. SOLID WASTE	
1. Estimated quantity of solid waste to be developed on the site	125 lbs./day
2. Method for disposal of solid waste	Town landfill
3. Plans for recycling of solid waste	none
I. BOAT YARDS, DOCKS, MARINAS	
1. Capacity of marina (number of boats, running feet)	n.a.
2. Description of docks and floats (site, dimensions)	n.a.
3. Description of sewage pumpout facilities (type of waste disposal)	n.a.
4. Description of fueling facilities and fuel storage tanks	n.a.

5. Description of fuel spill prevention measures and equipment — n.a.

J. IMPACT OF PROPOSED ACTION APPLIED FOR

1. Effects on plant species (upland and marine) — 11.5%removal

2. Effects on marine species (shellfish, finfish) — none

3. Effects on drainage and runoff — Slight decrease Trib. 2 All Storms. Slight increase to Trib. 1 for storms over 5 year intensity.

4. Effects on siltation of surface waters — Control measures on plans.

5. Effects on groundwater quality — Potential for slight degradation if salt used on road

6. Effects on surface water quality — Potential for slight degradation if salt used on road

K. ALTERNATIVES TO PROPOSED ACTION

1. Describe alternatives to the requested action — see below

2. Describe the benefits of the requested action over the alternatives — see below

The proposed design approximates the existing hyddrological environment. The slight increase in peak flow in tributary 1 for a period of ½ hour does not aggrivate existing flood conditions. Alternatives such as equal storage for this increase would require destruction of existing vegatation for no real benefit.

ARBORWOOD HIGHLANDS
ACTON, MASS.

SURFACE AND GROUNDWATER ANALYSIS

Prepared for:

NORTHEAST STRUCTURES & DEVELOPMENT INC.
BOX 215
BEDFORD, MASS.

COMMONWEALTH OF MASSACHUSETTS
BRUCE
M.
STAMSKI
No. 27400
REGISTERED
PROFESSIONAL ENGINEER

Prepared by:

JOSEPH W. MOORE COMPANY
16 RAILROAD AVE.
BEDFORD, MASS.

JANUARY 1978

JOB NO. M-318

SUBJECT: Drainage

LOCATION: Arborwood, Acton

CALC. BY: BMS
DATE: 1/23/72
CHECKED BY:
DATE:

JOSEPH W. MOORE Co.
LAND SURVEYORS AND CIVIL ENGINEERS

Drainage System Design

Drainage Area from 40 scale Topo plan

Planimeter sq in x 1600/43560 = acres

To CB-1

7.63 | 15.24 = 7.62 = .28 acres

3.55 | 7.26 = 3.63 = .13 acres

Combined Runoff Coefficient

.13 x .9 = .117
.15 x .3 = .045
.162/.28 = .58 = C

To CB-2

6.00 | 12.01 = 6.00 = .22

4.00 | 8.02 = 4.01 = .15 acres

Total = .50 acres .28

.28 x .9 = .252
.22 x .3 = .066
.318/.50 = .64

Time of Concentration Grass surface L=70' S = 10/70 = 14% Seeley nomograph = 7.5 min

100' of pavt @ S = 5% = .5

8 min to inlet CB-1

10 year design storm I @ 8 min Seeleys Fig B 10 year storm I = 1.8 on hour

From Fig J I = 5.5 in/hour @ 8 min

Size Drainlines for full flow see pg 7 of 7

Consideration for overall drainage impact consider:

1) Affect of Leaching Basins
2) Affect of Roof Drains to drywells
3) Affect of minor shift in drainage divide

CALC.
SHEET 1 OF 7 SHEETS

JOB NO. M-318

SUBJECT: Drainage

CALC. BY: BMS
DATE: 1/23/78
CHECKED BY:
DATE:

JOSEPH W. MOORE Co.
LAND SURVEYORS AND CIVIL ENGINEERS

LOCATION: Arborwood, Acton

Tributary 2 Flows under Fernwood

Area Shifted away from Tributary 2 (Flows across Fernwood Abutters Property)

1) Area Diverted by Road to Aborwood Drive Existing System

2.61
2 | 5.22
2.61 x 1600/43560 = .10 acres

existing c = .15
proposed c = 26x70' = .04 pavt @ .9 = .036
.06 grass @ .3 .018
.10 .054
c = .54

Note: Max 10year storm increase to existing drainage

Q = .10 x .54 x 8.5 = .3 cfs

very minor increase to existing CB's located 398' from proposed high point.

2) Area Diverted to Proposed drainage system

1.22 + 1.57
12.46 | 8.10

1.23 1.55 = 2.78 x 1600/43560 = .10

3) Roofs diverted to ground 26 x 65 x 2 = 3380/43560 = .08

Total Area diverted = .28 acres Decrease in AxC = .28 x .15 = .042

Increase in Runoff due to Drives 12x20 + 20x18 + 12x50 + 20x20 = 1600 = .0367

AxC .0367 x (.90 - .15) = .028 increase

Net effect decreases runoff potential
.042 - .028 = .014 decrease in AxC

CALC.
SHEET 2 OF 7 SHEETS

JOB NO. M-318

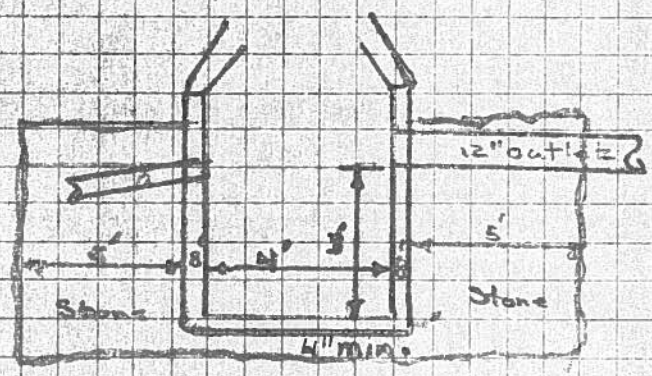
SUBJECT: Drainage

CALC. BY: BMS
DATE: 1/23/78
CHECKED BY:
DATE:

JOSEPH W. MOORE Co.
LAND SURVEYORS AND CIVIL ENGINEERS

LOCATION: Arborwood, Acton

Leaching Pit Capacity - prior to outflow



d = 3.67'

dia = 15.33

Bottom area = 184.0 SF

Side area = 15.33 x 3.14 x 3.67 = 176.7

Total = 361 SF

perc rate less than 2 min inch = 30"/hour = 2.5 FT/hour

outflow = 902.5 cf/hour

Total = 902.5 x 2 = 1805 cf/hour = .5 cfs

Drainage Area mostly the pavt area

.28 ac of pavt @ .9 = .252
.22 ac of grass @ .30 = .066
.32/.50 = .64 = C
.5 ac.

max rainfall intensity prior to outflow to swamp

Q = CIA

.5 = .64 x I x .5

I = 1.56"/hour

The system will not add to peak flows at time of concentration of receiving stream (pg 4) for storms of less than 5 year return interval.

1-24-78

CALC.
SHEET 3 OF 8 SHEETS

JOB NO. M-318

SUBJECT: Drainage

LOCATION: Arborwood, Acton

CALC. BY: BMS
DATE: 1/23/78
CHECKED BY:
DATE:

JOSEPH W. MOORE Co.
LAND SURVEYORS AND CIVIL ENGINEERS

Time of Concentration of Tributary 1

1500' initial overland flow $\Delta H = 30'$ kirpich nomo graph 10 x 2 = 20 min

1500' of stream flow @ 3 fps 8 min

1000' of swamp flow 250' wide

$S = 2'/1000' = .002$ $Q = 100$ cfs $n = .10$

$S^{1/2} = .644$

$R = \frac{A}{250}$

$Q = \frac{1.486}{n} A R^{2/3} S^{1/2}$

250'

$AR^{2/3} = 152.9$

if D = .75

$A \times \left(\frac{A}{250}\right)^{2/3} = 154.6$ OK

Edge of swamp at subdivision = 152.1

Flood elev = 152.85

A = 187.5

V = .55 fps

= 30 min

58 min

use 1 hour

Compute Runoff Coefficient

$Q_{10} = 50$ cfs $I_{10} = 1.8$ $c = .279$

$Q_{100} = 115$ cfs $I_{100} = 2.75$ $c = .421$

Rainfall I from US WB Tech Paper number 40

Tributatary I area 12.03 From 1" = 600 Town Parcels Map

124.12

12.06 = 99.66 acres = .1659 miles (Flood Hazard Study A = .17) close check

Area of Wetland

13.96

1.98 x 600 x 600 / 43560 = 16.4

Impervious Surface

70 buildings + 6,400 ft Road

70 x 30 x 50 = 105,000
6400 x 26 = 166400 } 271,400 = 6.2

1-26-78

CALC.
SHEET 4 OF 8 SHEETS

JOB NO. M-318

SUBJECT: Drainage

CALC. BY: BMS
DATE: 1/23/78
CHECKED BY:
DATE:

JOSEPH W. MOORE Co.
LAND SURVEYORS AND CIVIL ENGINEERS

LOCATION: Arborwood, Acton

C Factor for Land other Than impervious & swamp

$Q_{100} = 115$ $I_{100} = 2.75$ $A = 99.7$ acres

$Q = CIA$

$C = \frac{22.6 \times .9 + C \times 77.1}{99.7}$

$115 = \left(\frac{22.6 \times .9 + C \times 77.1}{99.7}\right) 2.75 \times 99.7$

$C = .278$ ← Work area of our site Hinckley Soils (Type A)
most of remaining highland in watershed
is type C soils (Hollis & Paxton soils)

The use 0.15 for existing runoff valid considering slopes and A soils on site.

Increase in overland Flow Drainage

1) Decrease due to house drywells = $3 \times 26 \times 65 = 5070$ SF = .11 acres.

2) Increase due to drives $18 \times 22 + 30 \times 12 + \frac{18+12}{2} \times 50 + 175 \times 12 + 40 \times 20 = 4406 \approx .10$

net increase in overland flow = $.10(.90 - .15) - (.11 \times .15) = .06 = +A \times C$

@ Peak flow in Brook increase = $Q_{10} = .06 \times 1.8 = .108$ cfs

$Q_{100} = .06 \times 2.75 = .165$ cfs

Increase due to Drainage system

$Q_{10} = (1.8 - 1.56) \times .64 \times .50 = .077$ cfs

$Q_{100} (2.75 - 1.56) \times .64 \times .50 = .381$ cfs

CALC.
SHEET 5 OF 8 SHEETS

JOB NO. M-312.

SUBJECT: Drainage

LOCATION: Arborwood, Acton

CALC. BY: BMS
DATE: 1/23/78
CHECKED BY:
DATE:

JOSEPH W. MOORE Co.
LAND SURVEYORS AND CIVIL ENGINEERS

Total increase in peak flow to Tributary 1

10 year storm = .108 + .077 = .185 cfs existing = 50 cfs % = 0.4%

100 year storm = .165 + .381 = .548 cfs existing = 115 cfs % = 0.5%

The .55 cfs increase in peak flow will have a negligible increase in flood stage. As the brook leaves site the flow path is

Area = 360 SF V = 115/360 = .32 fps

$(210 \times D) \times V = (.55)$

$D = \frac{.55}{210 \times .32} = .008'$ potential increase 100 year flood elev.

At arborwood road the Roadway Profile will flood 600' wide

The increase in peak flow will cause a $(600 \times D) \times V = Q$

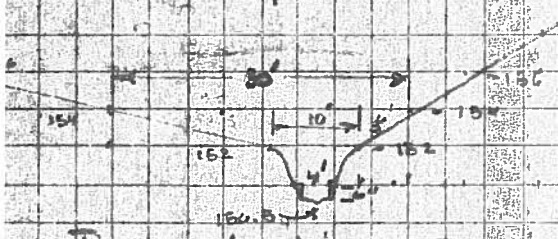
Area of flow = 700 SF V = .23 fps $600 \times D \times .23 = .55$

$\Delta D = .00398'$

The proposed increases in runoff is statistical in nature only. A .00398' increase would be unmeasurable in field

Flood elevation upper area of Trib 1. Swamp conveyance = 152.85 (pg 4)

At subdivision just down from proposed drain there is a natural restriction in flow path



$S = 2/60 = .033$ $S^{1/2} = .18$ n = .04 little brush, trees

@ 152 $Q = \frac{1.486}{.04} \times \left(\frac{10+4}{2} \times 1 + .5 \times 4\right) \times \left(\frac{9}{11.7}\right)^{2/3} \times .18 = 52.35$ cfs

@ 153 $Q = \frac{1.486}{.04} \times \left(9 + \frac{25.5+10}{2} \times 1\right) \times \left(\frac{26.75}{27.7}\right) \times .18 = 174.7$ cfs

The restriction causes backwater to 152.5 - Swamp conveyance governs elevation.

CALC.
SHEET 6 OF 8 SHEETS

JOB NO. M-13

SUBJECT: Drainage

CALC. BY: BMS
DATE: 1/31/78
CHECKED BY:
DATE:

JOSEPH W. MOORE Co.
LAND SURVEYORS AND CIVIL ENGINEERS

LOCATION: Arborwood, Acton

Note: This information previously on sheet 8 → copied over for clarity.

100 year Flows From Site @ Time of Peak Flow in Receiving Brooks

Existing

Area = 5.2 acres Time of Concentration Brook = 1 hour I_{100} = 2.75"/hour

Peak Q_{100} from site = 5.2 x .15 x 2.75 = 2.14 cfs

Volume after Peak

Time	Rainfall	Runoff Total	Runoff per time period
0			
	2.4 x 60 x 60/2 =	3852	3852
1			
			5492
2	3.3"/12 x .15 x 5.2 x 43560	9344	
			1416
3	3.8/12 x .15 x 5.2 x 43560	10760	
			2830
8	4.8"/12 x .15 x 5.2 x 43560	13590	

Proposed Condition note: lawns considered @ C=.15 due to underlying soils rapid drainage

1) Area Removed from surface runoff (houses) = 5@26 x 65/43560 = .194 acres
2) Area to Drainage System = .50 acres

Area now draining overland = 5.2 - .194 - .50 = 4.506
Area of drives (C=.90) = 1600 + 4406/43560 = .138 ac.

Overland drainage C .138 x .9 = .124
4.368 x .15 = .655
.779/4.506 = .173 = C

Q_{100} = .173 x 4.506 x 2.75 = 2.14 cfs Note: overland flow proposed is equal to existing
Potential increase is from street drainage

Street Drains 8 min time of concentration

1 hour peak .50 x .64 x 2.75 = .88 cfs Volume = .88 x 8 x 60/2 + .88 x 52 x 60 = 2956 - 1805 (infiltration in hour) = 1151 cf runoff
Δ = 3833 - 2956 = 877 cf - 1805 = 0 discharge.

2 hours 3.3"/12 x .50 x .64 x 43560 = 3833 total
Δ = 5444 - 3833 = 581 - 1805 = 0 discharge

3 hours = 3.8/12 x .50 x .64 x 43560 = 4414 total

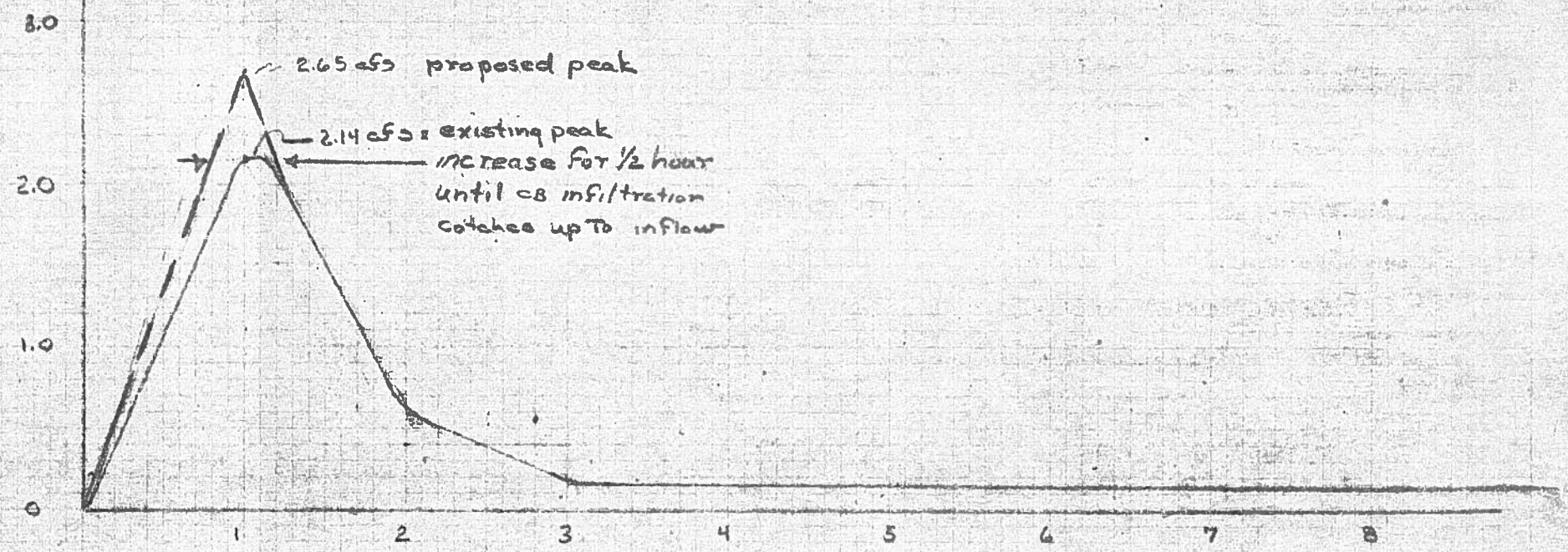
The infiltration capacity exceeds runoff after one hour.

CALC.
SHEET 8 OF 8 SHEETS

at time of peak on Receiving streams

The increase of 1.65 cfs will have a negligible effect on Flood Stage of Tributary 4.

100 year Hydrograph for total Flows from site.



JOB NO. M-318

SUBJECT: BMS

CALC. BY: BMS
DATE: 1/23/78
CHECKED BY:
DATE:

JOSEPH W. MOORE Co.
LAND SURVEYORS AND CIVIL ENGINEERS

LOCATION: Arborwood, Acton

Groundwater Considerations

Pollution of Groundwater

1) Sanitary sewage disposal fieldsets will be built 4'+ over water table or ledge. Under 3 of the 5 systems no groundwater encountered. Our preliminary tests in November found no groundwater on lots 3 & 4 subsequent tests encountered water @ 6.5 to 7.0' depth.

The septic tank effluent will not only be filtered by a 4' gravel layer but will travel latterally along ledge surface. There is very little chance of groundwater table contamination

Quantity of groundwater recharge

1) The leaching CB system for most storms will recharge groundwater table as existing site does.

2) Decrease due to drives = .0367 + .10 + .10 = .2367 acres

41" per year = 3.42'
- runoff .15(3.42) = .47
evaporation = 1.26
1.70' rainfall for recharge

Decrease = .2367 x 1.70 = .40 ac ft = 17528 cf/yr

3) Artificial Recharge by septic system
5 houses x 5 people x 80 GPD x 365/7.48 = 97594 cf/yr

80,000 CF increase/year

CALC. SHEET 1 OF 2 SHEETS

JOB NO. BMS

SUBJECT: BMS

CALC. BY: 1/23/78
DATE: 1/23/78
CHECKED BY:
DATE:

JOSEPH W. MOORE Co.
LAND SURVEYORS AND CIVIL ENGINEERS

LOCATION: Ashburwood, Acton

Total Recharge from Subdivision area.

1.70 x 5.2 acres = 8.84 = 38,970 cf/year

Affect of Increase

The 80,000 cf/year ÷ 219.2 cf/day = 9.1 cf/hour = .0025 cfs flow in Coles Brook ≈ 1gpm± (3/5 to Tributary One, 2/5 to Tributary Two)

The artifical recharge will add 1gpm± to flow of brooks

This will help maintain low flow in brooks.

CHECK 1-26-78

CALC.
SHEET 2 OF 2 SHEETS

JOB NO. M-318

SUBJECT: Soils

CALC. BY: BMS
DATE: 1/23/08
CHECKED BY:
DATE:

JOSEPH W. MOORE Co.
LAND SURVEYORS AND CIVIL ENGINEERS

LOCATION: Arborwood, Acton

Soils - Soil Logs on topo plan

USDA soil type - Hinckley

1) The onsite soils were found to be sand and gravel with 18" ± of organic over burden.

2) Ledge in some locations was at the surface. The depth to ledge in the primary work areas at a depth of 7.5' to over 10 feet.

3) Percolation rates rapid - under 2 min/inch most locations

4) No groundwater found on site during preliminary test pits in November. In Dec. after heavy rains groundwater encountered on lots 3 & 4. This probably due to backflow from wetland on Parcel A.

5) Sanitary Sewage Disposal Facilities are possible on the site.

CALC.
SHEET 1 OF 1 SHEETS

Sheet 7 of 8
Computed by: DWM Date: Jan.11,1978
Checked by: BMS Date: Jan23,1978

COMPUTATION FORM FOR DESIGN OF STORM SEWERS.

*10-Yr Frequency

City or Town: Acton
"Arborwood Drive
RCP n=.013
M-318

Structure No. From	Structure No. To	Length (Ft.)	Tributary Area (Acres) Increment	Tributary Area (Acres) Total	Time of Flow (Minutes) To Upper end	Time of Flow (Minutes) in Section	Runoff Coeff. "C"	"i" Rainfall Intensity (in./hr.)	"Q" Total Runoff (C.f.s.)	Slope of Conduit (ft./ft.)	Diam of pipe (in.)	Capacity Full (C.f.s.)	Velocity Full (f.p.s.)	Design Flow Velocity (f.p.s.)	Design Flow Velocity Head (ft.)	Design Flow Depth of Flow (ft.)	Design Flow Total Energy Head (ft.)	Manhole Invert Drop (ft.)	Fall in Conduit (ft.)	Drain Invert Elevation Upper End	Drain Invert Elevation Lower End	Ground Surface Upper End	Ground Surface Lower End
1 (c.b)	2 (c.b)	60'	.28	.28	8	–	0.58	5.5	.90	.005	12"	2.55	3.3	2.5	.10	.45	.55	–	0.30	167.50	167.20	171.66	171.66
2 (c.b)	3 (mH)	36'	.22	.50	8	–	0.64	5.5	1.76	.010	12"	3.56	4.56	3.6	.20	.50	.70	–	.36	167.20	166.82	171.66	172.5
3 (mH)	4 (out fall)	200'	–	.50	8		.64	5.5	1.76	.010	12"	3.56	4.56	3.6	.20	.50	.70	4.86	2.00	162.00	160.00	172.0	—

DAVID STANDLEY
COMMISSIONER

The Commonwealth of Massachusetts

Executive Office of Environmental Affairs

Department of Environmental Quality Engineering

Central Region Tel. 754-3226

75 Grove Street, Worcester, Massachusetts 01605

Rec'd 2/17/78

file DEQE #85-31

February 10, 1978

This Department received an application under General Laws Chapter 131, Section 40, on February 3, 1978 filed in the name of Northeast Structures and Development, Inc.

P.O. Box 215, Bedford, MA 01730

Owner of land Boudreau Family Trust, 41 Baskin Rd., Lexington, MA

This project is in the City/Town of Acton

and is located Arborwood Street

The following information is required

~~Notice of Intent ( )~~
~~Plans ( )~~
~~Locus Map ( )~~

This project has been designated by the File Number 85-31
Future correspondence regarding this project should bear this number.

This project ~~is~~/is not being forwarded to Waterways for evaluation under Chapters 21, 91 and 253.

Gilbert T. Joly
Gilbert T. Joly, P.E.
Regional Environmental Engineer

cc: Northeast Structures & Development, Inc.
P.O. Box 215
Bedford, MA 01730

Boudreau Family Trust
41 Baskin Road
Lexington, MA

P.S. If your project involves placing fill or structures in a stream, pond or wetland a Corps of Engineers permit may be required. For more information contact:

Department of the Army
New England Division, Corps of Engineers
424 Trapelo Road
Waltham, Mass. 02154
Attn: Morgan R. Rees, Chief, Regulatory Branch
Telephone: 617-894-2400, extension 332